REPORT
GEOTECHNICAL STUDY – UPDATE NO. 2
PROPOSED THE POINT EVENT CENTER
APPROXIMATELY 14000 SOUTH 200 WEST
DRAPER, UTAH (40.4934°, -111.8992°)

Submitted To:

Point Phase 1 Public Infrastructure District No. 1
c/o Wadsworth Development Group
166 East 14000 South
Draper, Utah 84020

Submitted By:

GSH Geotechnical, Inc.
473 West 4800 South
Salt Lake City, Utah 84123

April 23, 2026

Job No. 4152-001-25

April 23, 2026
Job No. 4152-001-25

Mr. Robert Booth
Point Phase 1 Public Infrastructure District No. 1
c/o Wadsworth Development Group
166 East 14000 South
Draper, Utah 84020

Mr. Booth:

Re: Report – Update No. 2
Geotechnical Study
The Point Event Center
Approximately 14000 South 200 West
Draper, Utah (40.4934°, -111.8992°)

1. INTRODUCTION

1.1 GENERAL

As requested by Mr. Shailesh Virlley of Gensler, this updated report presents the results of our geotechnical study performed at the site of The Point Event Center to be located at approximately 14000 South 200 West in Draper, Utah (40.4934°, -111.8992°). The general location of the site with respect to existing roadways, as of 2025, is presented on Figure 1, Vicinity Map. A more detailed layout of the site showing proposed facilities, existing roadways, and the borings drilled in conjunction with this study is presented on Figure 2, Site Plan.

1.2 OBJECTIVES AND SCOPE

The objectives and scope of the study were planned in discussions between Mr. Robert Booth of Wadsworth Development Group and Mr. Robert Gifford, P.G., of GSH Geotechnical, Inc. (GSH).

In general, the objectives of this study were to:

1. Define and evaluate the subsurface soil and groundwater conditions across the site.

GSH Geotechnical, Inc.
473 West 4800 South
Salt Lake City, Utah 84123
Tel: 801.685.9190
www.gshgeo.com

2. Provide appropriate foundation, earthwork, pavement, and geoseismic recommendations to be utilized in the design and construction of the proposed facilities.

In accomplishing these objectives, our scope has included the following:

1. A field program consisting of the exploration, logging, and sampling of 15 borings, and a stormwater percolation test.

2. A laboratory testing program.

3. An office program consisting of the correlation of available data, engineering analysis, and the preparation of this summary report.

1.3 AUTHORIZATION

Authorization was provided by returning a signed copy of the Professional Services Agreement No. 25-0724.rev1 dated August 1, 2025.

1.4 PROFESSIONAL STATEMENTS

Supporting data upon which our recommendations are based are presented in subsequent sections of this report. Recommendations presented herein are governed by the physical properties of the soils encountered in the exploration borings, projected groundwater conditions, and the layout and design data discussed in Section 2, Proposed Construction. If subsurface conditions other than those described in this report are encountered and/or if design and layout changes are implemented, GSH must be informed so that our recommendations can be reviewed and amended, if necessary.

Our professional services have been performed, our findings developed, and our recommendations prepared in accordance with generally accepted engineering principles and practices in this area at this time.

2. PROPOSED CONSTRUCTION

The 4.7-acre site is proposed to be developed with a 3- or 4-level, slab-on-grade 5,000-seat event venue structure with a footprint of 51,765 square-feet, an adjacent 2- or 3-story, slab-on-grade building with a footprint of 8,400 square feet, adjoining pavements, and public road. The main structure is anticipated to be constructed with steel and/or concrete tilt-up construction and be supported by conventional spread and continuous wall foundations.

Maximum real column and wall loads are anticipated to be up to 800 kips and 3 kips per lineal foot, respectively. Real loads are defined as the total of all dead plus frequently applied (reduced) live loads. Floor slab loads are anticipated to be approximately 150 pounds per square foot.

Paved parking areas, drive lanes, loading/unloading areas, and a public roadway are planned around the structures.

Projected traffic in the parking areas is anticipated to consist of a moderate volume of automobiles and light trucks, a light volume of medium-weight trucks, and occasional heavyweight trucks.

Projected traffic in the drive lanes, roadways, and loading/unloading areas are anticipated to consist of a moderate volume of automobiles, light trucks, and medium-weight trucks with a light volume of heavyweight trucks.

Site development will require some earthwork in the form of minor cutting and filling. At this time, we anticipate that maximum site grading cuts and fills, excluding utilities, will be on the order of 2 to 4 feet.

3. SITE INVESTIGATIONS

3.1 GENERAL

Subsurface conditions in unexplored locations or at other times may vary from those encountered at specific boring locations. If such variations are noted during construction or if project development plans are changed, GSH must review the changes and amend our recommendations, if necessary.

Boring locations were established by estimating distances and angles from site landmarks. If increased accuracy is desired by the client, we recommend that the boring locations and elevations be surveyed.

3.2 FIELD PROGRAM

To define and evaluate the subsurface soil and groundwater conditions across the site, 15 borings were completed within the accessible areas. These borings were completed to depths ranging from 5.5 to 71.5 feet with a truck-mounted drill rig equipped with hollow-stem augers. The approximate locations of the borings are presented on Figure 2. Additionally, a stormwater percolation test to determine the percolation rate was performed in Boring B-10 at a depth of 5.0 feet.

The field portion of our study was under the direct control and continual supervision of an experienced member of our geotechnical staff. During the course of the drilling operations, a continuous log of the subsurface conditions encountered was maintained. In addition, samples of the typical soils encountered were obtained for subsequent laboratory testing and examination. The soils were classified in the field based upon visual and textural examination. These classifications were supplemented by subsequent inspection and testing in our laboratory. Graphical representation of the subsurface conditions encountered is presented on Figures 3A through 3O, Boring Logs. Soils were classified in accordance with the nomenclature described on Figure 4, Key to Boring Log (USCS).

A 3.25-inch outside diameter, 2.42-inch inside diameter (Dames & Moore) and a 2.0-inch outside diameter, 1.38-inch inside diameter drive sampler (SPT) were utilized at select locations and depths. The blow counts recorded on the boring logs were those required to drive the sampler 12 inches with a 140-pound hammer dropping 30 inches.

Following completion of exploration operations, 1.25-inch diameter slotted PVC pipe was installed in Borings B-1 through B-6 to provide a means of monitoring the groundwater fluctuations. The borings were backfilled with auger cuttings.

3.3 LABORATORY TESTING

3.3.1 General

To provide data necessary for our engineering analysis, a laboratory testing program was performed. This program included moisture, density, partial gradation, Atterberg limits, consolidation, and chemical tests. The following paragraphs describe the tests and summarize the test data.

3.3.2 Moisture and Density Tests

To provide index parameters and to correlate other test data, moisture and density tests were performed on selected samples. The results of these tests are presented on the boring logs, Figures 3A through 3O.

3.3.3 Partial Gradation Tests

To aid in classifying the granular soils, partial gradation tests were performed. Results of the tests are tabulated below and presented on the boring logs, Figures 3A through 3O:

Boring No.	Depth (feet)	Percent Passing No. 200 Sieve	Moisture Content Percent	Soil Classification
B-3	0.5	69.8	2.4	CL
B-6	2.5	94.1	6.7	ML
B-6	20.0	70.1	13.6	CL
B-10	5.0	97.5	22.1	CL

3.3.4 Atterberg Limits Test

To aid in classifying the soils, Atterberg limits tests were performed on samples of the fine-grained cohesive soils. Results of the completed test are tabulated below and presented on the boring logs, Figures 3A through 3O:

Boring No.	Depth (feet)	Liquid Limit (percent)	Plastic Limit (percent)	Plasticity Index (percent)	Soil Classification
B-4	15.0	36	30	6	CL
B-4	35.0	33	26	7	CL
B-4	55.0	44	37	7	CL

3.3.5 Consolidation Tests

To provide data necessary for our settlement analysis, consolidation testing was performed on 5 representative samples of the natural fine-grained clay soils encountered at the site. The results of these tests indicate that the samples tested were moderately over-consolidated and will exhibit moderate strength and compressibility characteristics under the anticipated loading. Detailed results of the tests are maintained within our files and can be transmitted to you, upon your request.

3.3.6 Chemical Tests

To determine if the site soils will react detrimentally with concrete, chemical tests were performed on a representative sample of the near-surface soil encountered at the site. The results of the chemical tests are tabulated below:

Boring No.	Depth (feet)	Soil Classification	pH	Total Water-Soluble Sulfate (mg/kg-dry)
B-2	2.5	CL	8.2	33.0

4. SITE CONDITIONS

4.1 SURFACE

The site is located at approximately 14000 South 200 West in Draper, Utah (40.4934°, -111.8992°). The site was previously developed as the Utah State Prison, which has since been demolished, and the entirety of the site has been graded. The current topography of the site is relatively flat, grading down to the northeast with a total relief of approximately 4 feet. Site vegetation consists of various weeds.

The site is bounded in all directions by similar vacant, recently graded land. However, the overall site is bounded to the north by 13775 South Street and commercial structures beyond; to the east by Interstate I-15 followed by commercial structures and multi-family residential subdivisions beyond; to the south by 14600 South Street and Interstate I-15 followed by commercial structures and multi-family residential subdivisions; and to the west by commercial structures, a railroad, and a single-family residential subdivision beyond.

4.2 SUBSURFACE SOIL

The following paragraphs provide generalized descriptions of the subsurface profiles and soil conditions encountered within the borings conducted during this study. As previously noted, soil conditions may vary in unexplored locations.

The borings were completed to depths ranging from 5.5 to 71.5 feet. The soil conditions encountered in each of the borings, to the depths completed, were generally similar across the boring locations.

- Approximately 6 inches of topsoil was encountered in Borings B-1 and B-5. Topsoil thickness is frequently erratic and thicker zones of topsoil should be anticipated.

- Non-engineered fill soils were encountered in each boring to depths ranging between 6 inches to 2 feet beneath the existing ground surface. The non-engineered fill soils were loose/disturbed and primarily consisted of clay with varying sand and gravel content. However, GSH understands that the entirety of the site has undergone overall site grading, exposing re-worked natural soils at the surface.

- Natural soils were encountered below the ground surface in each of the borings. The natural soils consisted primarily of clay with varying silt and sand content, silt with varying clay and sand content and sand with varying silt and clay content.

The suitable natural clay soils consist of medium stiff to hard, slightly moist to saturated, brown, tan, orange, and gray in color, and moderately consolidated. Additionally, the natural clay soils are anticipated to exhibit moderate strength and compressibility characteristics under the anticipated loading.

The suitable natural sand (granular) soils consist of medium dense to very dense, slightly moist to saturated, and gray, brown, green, and yellow in color. The natural sand soils are anticipated to exhibit moderately high strength and moderately low compressibility characteristics under the anticipated load range.

For a more descriptive interpretation of subsurface conditions, please refer to Figures 3A through 3O, Boring Logs. The lines designating the interface between soil types on the boring logs

generally represent approximate boundaries. In situ, the transition between soil types may be gradual.

4.3 GROUNDWATER

On September 14, 2025 (11 days following drilling), groundwater was measured within the PVC pipes installed as tabulated below:

Boring No.	Groundwater Depth (feet)
	September 14, 2025
B-1	Dry to 16.5
B-2	28.0
B-3	29.0
B-4	27.0
B-5	23.5
B-6	26.8

Groundwater levels vary with changes in season and rainfall, construction activity, irrigation, snow melt, surface water run-off, and other site-specific factors.

5. DISCUSSIONS AND RECOMMENDATIONS

5.1 SUMMARY OF FINDINGS

The proposed structures may be supported upon conventional spread and continuous wall foundations supported upon suitable natural soils and/or structural fill extending to suitable natural soils.

The most significant geotechnical aspects at the site are:

1. The existing loose/disturbed soils at the surface and the potential to encounter additional non-engineered fill at the site.

2. The moderately consolidated clays encountered within the subsurface at the site.

Prior to proceeding with construction, removal of any existing debris, surface vegetation, root systems, topsoil, non-engineered fill, and any deleterious materials from beneath an area extending

out at least 5 feet from the perimeter of the proposed structure footprints and 3 feet beyond pavements and exterior flatwork areas will be required. All existing utility locations should be reviewed to assess their impact on the proposed construction and abandoned and/or relocated as appropriate.

Due to the developed nature of this site and the surrounding area, additional non-engineered fills may exist in unexplored areas of the site. Based on our experience, non-engineered fills are frequently erratic in composition and consistency. All surficial loose/disturbed soils and non-engineered fills must be removed below all footings, floor slabs, and pavements. The in situ, non-engineered fills may remain below flexible pavements if free of any deleterious materials, of limited thickness, and if properly prepared, as discussed later in this report. Even with proper preparation, pavements established overlying non-engineered fills may encounter some long-term movements unless the non-engineered fills are completely removed.

Due to moderately consolidated clay soils encountered at the site, granular structural replacement fill will be required below more heavily loading footings, see section 5.4.3, Settlements of this report for more detailed information.

Proof rolling of the natural clay subgrade must not be completed if cuts extend to within 1 foot of the groundwater surface. In areas where cuts are to extend to within 1 foot of the groundwater surface, stabilization must be anticipated.

To reduce disturbance of the natural soils during excavation, it is recommended that low-impact, track-mounted equipment with smooth edge buckets/blades be utilized.

Detailed discussions pertaining to earthwork, foundations, pavements, and the geoseismic setting of the site are presented in the following sections.

5.2 EARTHWORK

5.2.1 Site Preparation

Initial site preparation will consist of the removal of any existing debris, non-engineered, surface vegetation, root systems, topsoil, and any deleterious materials from beneath an area extending out at least 5 feet from the perimeter of the proposed structure footprint and 3 feet beyond pavements and exterior flatwork areas. All existing utility locations should be reviewed to assess their impact on the proposed construction and abandoned and/or relocated as appropriate.

In situ, non-engineered fills may remain below flexible pavements if free of debris and deleterious materials, less than 3 feet in thickness, and if properly prepared. Proper preparation below pavements will consist of the scarification of the upper 12 inches below asphalt concrete (flexible pavement), followed by moisture preparation and re-compaction to the requirements of structural

fill. Even with proper preparation, pavements established overlying non-engineered fills may encounter some long-term movements unless the non-engineered fills are completely removed.

It must be noted that from a handling and compaction standpoint, soils containing high amounts of fines (silts and clays) are inherently more difficult to rework and are very sensitive to changes in moisture content, requiring very close moisture control during placement and compaction. This will be very difficult, if not impossible, during wet and cold periods of the year. Additionally, the on-site soils are likely above optimum moisture content for compacting at present and would require some drying prior to re-compacting.

Subsequent to stripping and prior to the placement of floor slabs, foundations, structural site grading fills, exterior flatwork, and pavements, the exposed subgrade must be proof rolled by passing moderate-weight rubber tire-mounted construction equipment over the surface at least twice. If excessively soft or otherwise unsuitable soils are encountered, they must be completely removed. If removal depth required is greater than 2 feet below footings, GSH must be notified to provide further recommendations.

Subgrade preparation as described must be completed prior to placing overlying structural site grading fills.

To reduce disturbance of the natural soils during excavation, it is recommended that low-impact, track-mounted equipment with smooth edge buckets/blades be utilized.

GSH must be notified prior to the placement of structural site grading fills, floor slabs, footings, and pavements to verify that all loose/disturbed soils and non-engineered fills have been completely removed and/or properly prepared.

5.2.2 Temporary Excavations

Temporary excavations up to 8 feet deep in fine-grained cohesive soils, above or below the water table, may be constructed with sideslopes no steeper than one-half horizontal to one vertical (0.5H:1.0V). Excavations deeper than 8 feet are not anticipated at the site.

For granular (cohesionless) soils, construction excavations above the water table, not exceeding 4 feet, shall be no steeper than one-half horizontal to one vertical (0.5H:1.0V). For excavations up to 8 feet, in granular soils and above the water table, the slopes shall be no steeper than one horizontal to one vertical (1H:1V). Excavations encountering saturated cohesionless soils will be very difficult and will require very flat sideslopes and/or shoring, bracing, and dewatering.

To reduce disturbance of the natural soils during excavation, it is recommended that low-impact, track-mounted equipment with smooth edge buckets/blades be utilized.

All excavations must be inspected periodically by qualified personnel. If any signs of instability or excessive sloughing are noted, immediate remedial action must be initiated.

5.2.3 Structural Fill

Structural fill is defined as all fill which will ultimately be subjected to structural loadings, such as imposed by footings, floor slabs, pavements, etc. Structural fill will be required as backfill over foundations and utilities, as site grading fill, and as replacement fill below footings. All structural fill must be free of surface vegetation, root systems, rubbish, topsoil, frozen soil, and other deleterious materials.

Structural site grading fill is defined as structural fill placed over relatively large open areas to raise the overall grade. For structural site grading fill, the maximum particle size shall not exceed 4 inches; although, occasional larger particles, not exceeding 8 inches in diameter, may be incorporated if placed randomly in a manner such that "honeycombing" does not occur, and the desired degree of compaction can be achieved. The maximum particle size within structural fill placed within confined areas shall be restricted to 2 inches.

On-site soils may be re-utilized as structural site grading fill if they do not contain construction debris or deleterious material and meet the requirements of structural fill. Fine-grained soils will require very close moisture control and may be very difficult, if not impossible, to properly place and compact during wet and cold periods of the year.

Imported structural fill below foundations and floor slabs shall consist of a well graded sand and gravel mixture with less than 30 percent retained on the three-quarter-inch sieve and less than 20 percent passing the No. 200 Sieve (clays and silts).

To stabilize soft subgrade conditions (if encountered) or where structural fill is required to be placed closer than 2.0 feet above the water table at the time of construction, a mixture of coarse angular gravels and cobbles and/or 1.5- to 2.0-inch gravel (stabilizing fill) shall be utilized. It may also help to utilize a stabilization geosynthetic, such as Mirafi RS580i or Tensar NX850, placed on the natural ground if 1.5- to 2.0-inch gravel is used as stabilizing fill.

5.2.4 Fill Placement and Compaction

All structural fill shall be placed in lifts not exceeding 8 inches in loose thickness. Structural fills shall be compacted in accordance with the percent of the maximum dry density as determined by the AASHTO[1] T180 (ASTM[2] D1557) compaction criteria in accordance with the table on the following page.

[1] American Association of State Highway and Transportation Officials
[2] American Society for Testing and Materials

Location	Total Fill Thickness (feet)	Minimum Percentage of Maximum Dry Density
Beneath an area extending at least 5 feet beyond the perimeter of the structure	0 to 5	95
Site grading fills outside area defined above	0 to 5	90
Utility trenches within structural areas	--	96
Road base	--	96

Structural fills greater than 5 feet thick are not anticipated at the site.

Subsequent to stripping and prior to the placement of structural site grading fill, the subgrade shall be prepared as discussed in Section 5.2.1, Site Preparation, of this report. In confined areas, subgrade preparation shall consist of the removal of all loose or disturbed soils.

Coarse angular gravel and cobble mixtures (stabilizing fill), if utilized, shall be end dumped, spread to a maximum loose lift thickness of 15 inches, and compacted by dropping a backhoe bucket onto the surface continuously at least twice. As an alternative, the stabilizing fill may be compacted by passing moderately heavy construction equipment or large self-propelled compaction equipment over the surface at least twice. Subsequent fill material placed over the coarse gravels and cobbles shall be adequately compacted so that the "fines" are "worked into" the voids in the underlying coarser gravels and cobbles. Where soil fill materials are to be placed directly over more than about 18 inches of clean gravel, a separation geofabric, such as Mirafi 140N or equivalent, is recommended to be placed between the gravel and subsequent soil fills.

Non-structural fill may be placed in lifts not exceeding 12 inches in loose thickness and compacted by passing construction, spreading, or hauling equipment over the surface at least twice.

5.2.5 Utility Trenches

All utility trench backfill material below structurally loaded facilities (footings, floor slabs, flatwork, pavements, etc.) shall be placed at the same density requirements established for structural fill. If the surface of the backfill becomes disturbed during the course of construction, the backfill shall be proof rolled and/or properly compacted prior to the construction of any exterior flatwork over a backfilled trench. Proof rolling shall be performed by passing moderately loaded rubber tire-mounted construction equipment uniformly over the surface at least twice. If excessively loose or soft areas are encountered during proof rolling, they shall be removed to a maximum depth of 2 feet below design finish grade and replaced with structural fill.

Many utility companies and City-County governments are now requiring that Type A-1a or A-1b (AASHTO Designation – granular soils with limited fines) soils be used as backfill over utilities. These organizations are also requiring that in public roadways, the backfill over major utilities be compacted over the full depth of fill to at least 96 percent of the maximum dry density as determined by the AASHTO T180 (ASTM D1557) method of compaction. GSH recommends that as the major utilities continue onto the site that these compaction specifications are followed.

Fine-grained soils, such as silts and clays, are not recommended for utility trench backfill in structural areas.

To reduce disturbance of the natural soils during excavation, it is recommended that low-impact, track-mounted equipment with smooth edge buckets/blades be utilized.

5.3 GROUNDWATER

On September 14, 2025 (11 days following drilling), groundwater was measured within the PVC pipes installed as tabulated below:

Boring No.	Groundwater Depth (feet)
	September 14, 2025
B-1	Dry to 16.5
B-2	28.0
B-3	29.0
B-4	27.0
B-5	23.5
B-6	26.8

The groundwater measurements presented are conditions at the time of the field exploration and may not be representative of other times or locations. Groundwater levels may vary seasonally and with precipitation, as well as other factors including irrigation. Evaluation of these factors is beyond the scope of this study. Groundwater levels may, therefore, be at shallower or deeper depths than those measured during this study, including during construction and over the life of the structure.

The extent and nature of any dewatering required during construction will be dependent on the actual groundwater conditions prevalent at the time of construction and the effectiveness of construction drainage to prevent run-off into open excavations.

5.3.1 Stormwater Percolation Test

A stormwater percolation test was performed at a depth of approximately 5 feet in the representative natural clay soils at Boring B-10. The field percolation rate was 45 minutes per inch. This design percolation rate should be considered typical for the soils at the site.

5.4 SPREAD AND CONTINUOUS WALL FOUNDATIONS ESTABLISHED UPON SUITABLE NATURAL SOILS

5.4.1 Design Data

The results of our analysis indicate that the proposed structures may be supported upon conventional spread and continuous wall foundations established upon suitable natural soils and/or structural fill extending to suitable natural soils. Under no circumstances shall foundations be established over non-engineered fills, loose or disturbed soils, topsoil, surface vegetation, root systems, rubbish, construction debris, other deleterious materials, frozen soils, or within ponded water. More heavily loaded footings will require a certain amount of granular structural replacement fill as specified in Section 5.4.3, Settlements, of this report. For design, the following parameters are provided:

Minimum Recommended Depth of Embedment for Frost Protection	- 30 inches
Minimum Recommended Depth of Embedment for Non-frost Conditions	- 15 inches
Recommended Minimum Width for Continuous Wall Footings	- 18 inches
Minimum Recommended Width for Isolated Spread Footings	- 24 inches
Recommended Net Bearing Capacity for Real Load Conditions	- 2,000 pounds per square foot
Bearing Capacity Increase for Seismic Loading	- 50 percent

The term “net bearing capacity” refers to the allowable pressure imposed by the portion of the structure located above lowest adjacent final grade. Therefore, the weight of the footing and backfill to lowest adjacent final grade need not be considered. Real loads are defined as the total

of all dead plus frequently applied live loads. Total load includes all dead and live loads, including seismic and wind.

5.4.2 Installation

Under no circumstances shall the footings be installed upon, non-engineered fills, loose or disturbed soils, topsoil, surface vegetation, root systems, rubbish, construction debris, or other deleterious materials. If unsuitable soils are encountered, they must be removed and replaced with compacted granular fill. If granular soils become loose or disturbed, they must be recompacted prior to pouring the concrete.

The width of structural replacement fill below footings shall be equal to the width of the footing plus one foot for each foot of fill thickness.

5.4.3 Settlements

Granular structural replacement fill will be required under more heavily loaded footings. For the required amount, refer to the table below:

Foundations	Loading	Minimum Thickness of Replacement Structural Granular Fill (feet)
Wall	Up to 3 kips per lineal foot	0
Spread	Up to 200 kips	0
	200 kips to 350 kips	1.0
	350 kips to 500 kips	2.0
	500 kips to 650 kips	3.0
	650 kips to 800 kips	4.0

Based on column loadings, soil bearing capacities, and the foundation recommendations as discussed above, we expect primary total settlement beneath individual foundations to be less than one inch.

The amount of differential settlement is difficult to predict because the subsurface and foundation loading conditions can vary considerably across the site. However, we anticipate differential settlement between adjacent foundations could vary from 0.5 to 0.75 inch. The final deflected shape of the structure will be dependent on actual foundation locations and loading.

5.5 SPREAD AND CONTINUOUS WALL FOUNDATIONS ESTABLISHED UPON RAMMED-AGGREGATE PIERS

5.5.1 General

Due to the depths of non-engineered fills at the site, GSH also evaluated the use of rammed-aggregate piers to bypass the non-engineered fills. Rammed-aggregate piers will require less earthwork and increase the bearing capacity of the natural soils.

If rammed-aggregate piers are used, then in situ fill soils may remain in place below floor slabs if properly prepared. Proper preparation of fills below floor slabs shall consist of the temporary removal of the upper 12 inches, followed by the scarification of the underlying 12 inches, followed by moisture preparation and re-compaction to the requirements of structural fill. The previously removed 12 inches of fills may be re-compacted and placed as structural fill.

5.5.2 Design Data

Rammed-aggregate piers soil reinforcement elements are constructed by drilling a 24- or 30-inch diameter hole and then building a bottom bulb of clean, open-graded stone using a beveled, high-energy tamper. The rammed-aggregate piers shaft is constructed on top of the bottom bulb using well graded highway base course stone placed in thin lifts (12 inches compacted thickness). The result is a reinforced zone of soil directly under footings that allows for the construction of shallow spread footings proportioned for a relatively high bearing pressure. Rammed-aggregate piers elements are spaced singly under continuous footings or in close groups to support concentrated column loads.

Rammed-aggregate piers soil reinforcement is a design/build element and must be designed and constructed by a licensed installer. The installer should provide layout and detailed design calculations sealed by a professional engineer licensed in the State of Utah. The design calculations should demonstrate that rammed-aggregate piers soil reinforcement is designed to control settlement to magnitudes within the criteria for this project.

For the design of conventional spread and continuous wall foundation constructed over rammed-aggregate piers elements, the following parameters are provided:

Minimum Recommended Depth of Embedment for Frost Protection	- 30 inches
Minimum Recommended Depth of Embedment for Non-frost Conditions	- 15 inches
Recommended Minimum Width for Continuous Wall Footings	- 16 inches

Minimum Recommended Width for Isolated Spread Footings	- 30 inches
Bearing Capacity for Footings Overlying Rammed-Aggregate Piers	- Approximately 4,000 to 7,000 pounds per square foot*

*To be developed as design/build by rammed-aggregate piers licensed installer.

The term "net bearing capacity" refers to the allowable pressure imposed by the portion of the structure located above lowest adjacent final grade. Therefore, the weight of the footing and backfill to lowest adjacent final grade need not be considered. Real loads are defined as the total of all dead plus frequently applied live loads. Total load includes all dead and live loads, including seismic and wind.

5.5.3 Installation

Foundations must be established directly upon the undisturbed tops of the pier systems. It is recommended that prior to installing rammed-aggregate piers, all site grading activities be completed.

Unsuitable soils shall be completely removed beneath footings. Under no circumstances shall the footings be installed overlying loose/disturbed soil, deleterious materials, frozen soil, or within ponded water.

5.5.4 Settlements

Maximum settlements of foundations designed and installed over rammed-aggregate piers should be less than one-half inch. However, these estimates will be refined with the design of the system.

5.6 LATERAL RESISTANCE

Lateral loads imposed upon foundations due to wind or seismic forces may be resisted by the development of passive earth pressures and friction between the base of the footings and the supporting soils. In determining frictional resistance, a coefficient of friction of 0.35 may be utilized for the footing interface with in situ natural clay soils and 0.40 for footing interface with natural granular soils or granular structural fill. Passive resistance provided by properly placed and compacted granular structural fill above the water table may be considered equivalent to a fluid with a density of 300 pounds per cubic foot. Below the water table, this granular soil shall be considered equivalent to a fluid with a density of 150 pounds per cubic foot.

A combination of passive earth resistance and friction may be utilized provided that the friction component of the total is divided by 1.5.

5.7 LATERAL PRESSURES

Parameters, as presented within this section, are for backfills which will consist of drained soil placed and compacted in accordance with the recommendations presented herein.

The lateral pressures imposed upon subgrade facilities will, therefore, be basically dependent upon the relative rigidity and movement of the backfilled structure. For active walls, such as retaining walls which can move outward (away from the backfill), drained backfill may be considered equivalent to a fluid with a density of 40 pounds per cubic foot in computing lateral pressures. For more rigid subgrade walls that are not more than 10 inches thick, granular backfill may be considered equivalent to a fluid with a density of 50 pounds per cubic foot. For very rigid non-yielding walls, granular backfill shall be considered equivalent to a fluid with a density of at least 60 pounds per cubic foot. The above values assume that the surface of the soils slope behind the wall is horizontal and that the granular fill within 3 feet of the wall will be compacted with hand-operated compacting equipment.

For seismic loading of below-grade walls, the uniform lateral pressures below, in pounds per square foot (psf), shall be added based on wall depth and wall case:

Uniform Lateral Pressures			
Wall Height (Feet)	**Active Pressure Case (psf)**	**Moderately Yielding Case (psf)**	**At Rest/Non-Yielding Case (psf)**
4	40	69	97
6	56	101	146
8	74	135	195
10	93	168	244

5.8 FLOOR SLABS

Floor slabs may be established upon suitable natural subgrade soils or structural fill extending to suitable natural soils. Under no circumstances shall floor slabs be established directly over non-engineered fills, loose or disturbed soils, sod, rubbish, construction debris, other deleterious materials, frozen soils, or within ponded water.

To facilitate curing of the concrete and to provide a capillary moisture break, it is recommended that floor slabs be directly underlain by at least 4 inches of "free-draining" fill, such as "pea" gravel or three-quarters to one inch minus clean gap-graded gravel.

GSH understands that a vapor barrier is proposed to be placed directly beneath the floor slab. GSH understands that the floor slab subgrade specifications are to be designed by others.

Settlement of lightly loaded floor slabs designed according to previous recommendations (average uniform pressure of 200 pounds per square foot or less) is anticipated to be less than one-quarter of an inch.

The natural clay soils have a subgrade modulus of 100 pounds per cubic inch.

5.9 PAVEMENTS

The natural clay and silt soils and non-engineered fills will exhibit poor pavement support characteristics when saturated. All pavement areas must be prepared as previously discussed (see Section 5.2.1, Site Preparation). Under no circumstances shall pavements be established over unprepared non-engineered fills, loose or disturbed soils, topsoil, surface vegetation, root systems, rubbish, construction debris, other deleterious materials, frozen soils, or within ponded water. Even with proper preparation, pavements established overlying non-engineered fills may encounter some long-term movements unless the non-engineered fills are completely removed. With the subgrade soils and the estimated projected traffic as discussed in Section 2, Proposed Construction, the following pavement sections are recommended:

Parking Areas

(Light Volume of Automobiles and Light Trucks,
Occasional Medium-Weight Trucks,
and No Heavyweight Trucks)
[8 equivalent 18-kip axle loads per day]

Flexible Pavements:
(Asphalt Concrete)

3.0 inches	Asphalt concrete
8.0 inches	Aggregate base (roadbase)
Over	Properly prepared natural subgrade soils and/or structural site grading fill extending to properly prepared natural subgrade soils

Rigid Pavements:
(Non-reinforced Concrete)

5.0 inches	Portland cement concrete (non-reinforced)
5.0 inches	Aggregate base (roadbase)
Over	Properly prepared and stabilized natural subgrade soils, and/or structural site grading fill extending to properly prepared natural subgrade soils

Drive Lanes and Loading/Unloading Areas

(Moderate Volume of Automobiles and Light Trucks,
Light Volume of Medium-Weight Trucks,
and Occasional Heavyweight Trucks)
[28 equivalent 18-kip axle loads per day]

Flexible Pavements:
(Asphalt Concrete)

4.0 inches	Asphalt concrete
8.0 inches	Aggregate base (roadbase)
Over	Properly prepared natural subgrade soils and/or structural site grading fill extending to properly prepared natural subgrade soils

Rigid Pavements:
(Non-reinforced Concrete)

6.0 inches	Portland cement concrete (non-reinforced)
6.0 inches	Aggregate base (roadbase)
Over	Properly prepared and natural subgrade soils, and/or structural site grading fill extending to properly prepared natural subgrade soils

Public Roadways*

(High Volume of Automobiles (1,500), Light Trucks (500),
And a Moderate Volume of Medium-Weight Trucks (22),
and Heavyweight Trucks(10))
[80 equivalent 18-kip axle loads per day]

Flexible Pavements:
(Asphalt Concrete)

4.0 inches	Asphalt concrete
8.0 inches	Aggregate base (roadbase)
10.0 inches	Subbase**
Over	Properly prepared natural subgrade soils and/or structural site grading fill extending to properly prepared natural subgrade soils

Rigid Pavements:
(Non-reinforced Concrete)

8.0 inches	Portland cement concrete (non-reinforced)
8.0 inches	Aggregate base (roadbase)
8.0 inches	Subbase
Over	Properly prepared and stabilized natural subgrade soils, and/or structural site grading fill extending to properly prepared natural subgrade soils

* GSH must verify that the subgrade in the proposed public roadway areas has a CBR of 5 percent or greater. GSH also recommends that the anticipated traffic counts be confirmed.
**Subbase must meet a California Bearing Ratio (CBR) of 30 percent or greater.

For dumpster pads, we recommend a pavement section consisting of 8.0 inches of Portland cement concrete, 12.0 inches of aggregate base, over properly prepared natural subgrade or site grading

structural fills. Dumpster pads shall not be constructed overlying non-engineered fills under any circumstances.

The above rigid pavement sections are for non-reinforced Portland cement concrete. Concrete shall be designed in accordance with the American Concrete Institute (ACI) and joint details shall conform to the Portland Cement Association (PCA) guidelines. The concrete shall have a minimum 28-day unconfined compressive strength of 4,000 pounds per square inch and contain 6 percent ±1 percent air-entrainment.

The road base must conform to applicable sections of the current Utah Department of Transportation (UDOT) Standard Specifications. All asphalt material and paving operations shall meet applicable specifications of the Asphalt Institute and UDOT. A GSH technician shall observe placement and perform density testing of the road base material and asphalt. Please note that the 3rd column of Table 2 of Section 02721 "Job Mix Gradation Tolerance" does not apply to road base utilized on this project and all road base must meet the requirements of the 2nd column of Table 2 of Section 02721 "Job Mix Gradation Target Blend" without any additional tolerances. The aggregate gradation limits presented on Table 2 of Section 02721, Untreated Base Course (UTBC) (road base) of UDOT 2025 Standard Specification for Road and Bridge Construction are tabulated below:

Gradation Limits	
Sieve Size	**Job Mix Gradation Target Blend**
1 1/2 inch	100
1 inch	90-100
3/4 inch	70-85
1/2 inch	65-80
3/8 inch	55-75
No. 4	40-65
No. 16	25-40
No. 200	7-11

Percent passing based on total aggregate (dry weight), and fine and coarse aggregate having approximately the same bulk specific gravities.

Please note that the recommended pavement section is based on estimated post-construction traffic loading. If the pavement is to be constructed and utilized by construction traffic, the above pavement section may prove insufficient for heavy truck traffic, such as concrete trucks or tractor-trailers used for construction delivery. Unexpected distress, reduced pavement life, and/or premature failure of the pavement section could result if subjected to heavy construction traffic and the owner should be

made aware of this risk. If the estimated traffic loading stated herein is not correct, GSH must review actual pavement loading conditions to determine if revisions to these recommendations are warranted.

5.10 CEMENT TYPES

The laboratory tests indicate that the natural soils tested contain a negligible amount of water-soluble sulfates. Based on our test results, concrete in contact with the on-site soil will have a low potential for sulfate reaction (ACI 318, Table 4.3.1). Therefore, all concrete which will be in contact with the site soils may be prepared using Type I or IA cement.

5.11 GEOSEISMIC SETTING

5.11.1 General

Utah municipalities have adopted the International Building Code (IBC) 2021. The IBC 2021 code refers to ASCE 7-16 Minimum Design Loads and Associated Criteria for Buildings and Other Structures (ASCE 7-16) determines the seismic hazard for a site based upon mapping of bedrock accelerations prepared by the United States Geologic Survey (USGS) and the soil site class. The USGS values are presented on maps incorporated into the IBC code and are also available based on latitude and longitude coordinates (grid points).

5.11.2 Faulting

Based on our review of available literature, no active faults pass through or immediately adjacent to the site. The nearest active mapped fault consists of the Salt Lake City Section of the Wasatch Fault, located about 3.3 miles to the east of the site.

5.11.3 Site Class

For dynamic structural analysis, the Site Class D – Default Soil Profile as defined in Chapter 20 of ASCE 7-16 (per Section 1613.3.2, Site Class Definitions, of IBC 2021) can be utilized. If a measured site class is desired based on the project structural engineer's evaluation and recommendations, additional testing and analysis can be completed by GSH to determine the measured site class. Please contact GSH for additional information.

5.11.4 Ground Motions

The IBC 2021 code is based on USGS mapping, which provides values of short and long period accelerations for average bedrock values for the Western United States and must be corrected for local soil conditions. The following table summarizes the peak ground and short and long period accelerations for the MCE event and incorporates the appropriate soil amplification factor for a Site Class D – Default* Soil Profile. Based on the site latitude and longitude (40.4934 degrees

north and 111.8992 degrees west, respectively) and Risk Category II, the values for this site are tabulated below:

Spectral Acceleration Value, T	Bedrock Boundary [mapped values] (% g)	Site Coefficient	Site Class D - Default* [adjusted for site class effects] (% g)	Design Values** (% g)
0.2 Seconds (Short Period Acceleration)	S_S = 144.6	F_a = 1.200	S_{MS} = 173.5	S_{DS} = 115.7
1.0 Second (Long Period Acceleration)	S_1 = 53.1	F_v = 1.769	S_{M1} = 93.9	S_{D1} = 62.6

* If a measured site class in accordance with IBC 2021/ASCE 7-16 is beneficial based on the project structural engineer's review, please contact GSH for additional options for obtaining this measured site class.

**IBC 2021/ASCE 7-16 may require a site-specific study based on the project structural engineer's evaluation and recommendations. If needed, GSH can provide additional information and analysis including a complete site-specific study in accordance with chapter 21 of ASCE 7-16.

5.11.5 Liquefaction

The site is located in an area that has been identified by the Utah Geological Survey (UGS) as being a "high" liquefaction potential zone. Liquefaction is defined as the condition when saturated, loose, granular soils lose their support capabilities because of excessive pore water pressure, which develops during a seismic event. Clayey soils, even if saturated, will generally not liquefy during a major seismic event.

Calculations were performed using the procedures described in the 2014 Soil Liquefaction During Earthquakes Monograph by Idriss and Boulanger[3]. Our calculations indicate the medium dense, saturated sand layers encountered in Borings B-3 and B-6 could liquefy during the design seismic event. Due to the clayey nature of the soils, liquefaction is not anticipated to occur within the soils encountered at this site. Calculated settlement associated with the liquefaction of each layer within the borings was less than 0.5 inches. This magnitude of settlement must be evaluated by the structural engineer to design for life safety. Additionally, lateral spread and ground rupture are unlikely to occur.

[3] Idriss, I. M., and Boulanger, R. W. (2014), Soil liquefaction during earthquakes: Monograph MNO-12, Earthquake Engineering Research Institute, Oakland, CA, 261 pp.

5.12 SITE VISITS

GSH must verify that all topsoil/disturbed soils and any other unsuitable soils have been removed, that non-engineered fills have been removed and/or properly prepared, and that suitable soils have been encountered prior to placing site grading fills, footings, slabs, and pavements. Additionally, GSH must observe fill placement and verify in-place moisture content and density of fill materials placed at the site.

6. CLOSURE

If you have any questions or would like to discuss these items further, please feel free to contact us at (801) 685-9190.

Respectfully submitted,

GSH Geotechnical, Inc.

Tristen Leberknight
Staff Engineer, E.I.T.

Reviewed by:

Alan D. Spilker, P.E.
State of Utah No. 334228
President/Senior Geotechnical Engineer

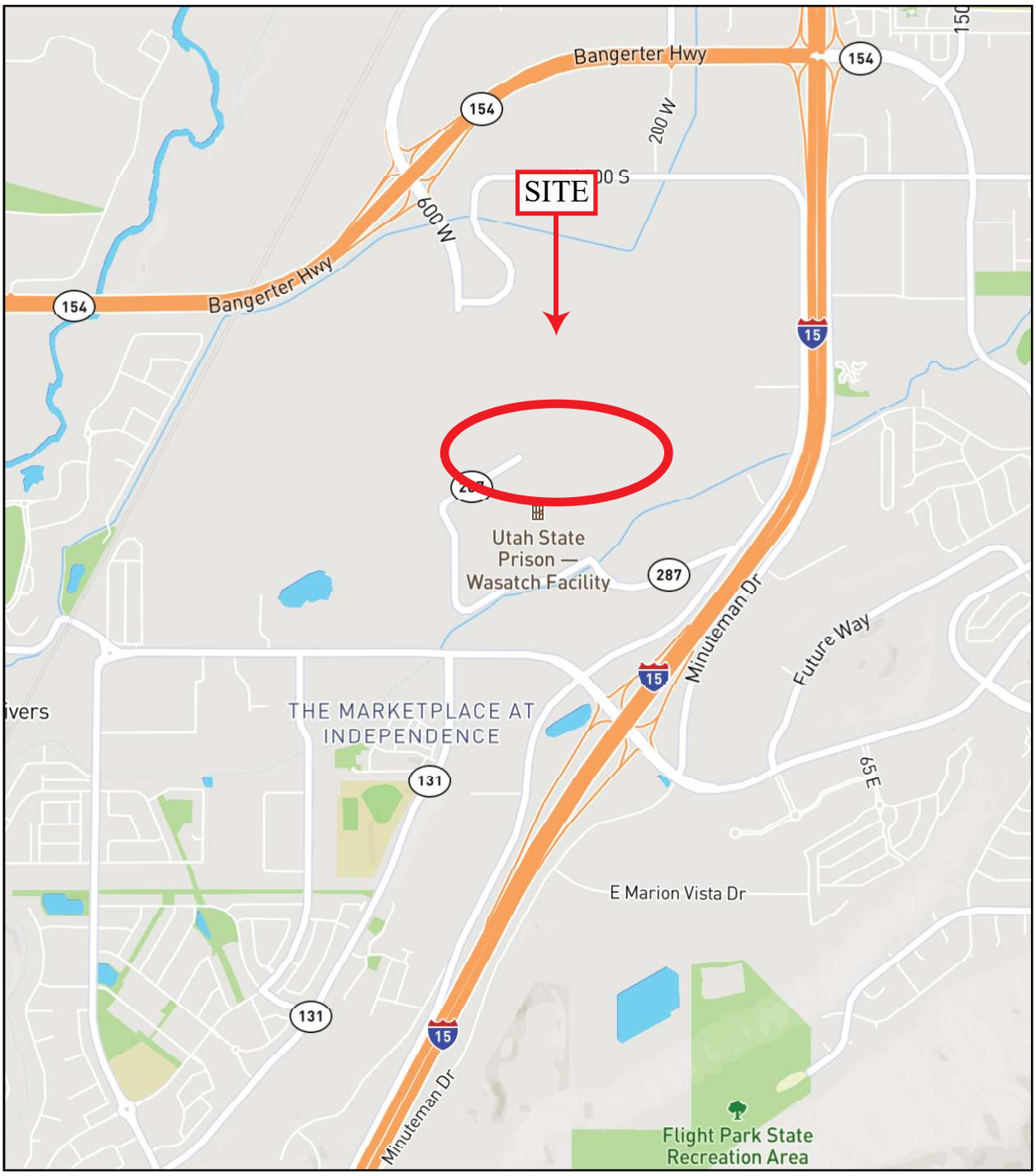
TL/ADS:jmt

Encl.
Figure	1,	Vicinity Map
Figure	2,	Site Plan
Figures	3A	through 3O, Boring Logs
Figure	4,	Key to Boring Log (USCS)
Attachment	1,	Bearing Capacity and Settlement Calculations

Addressee: (email)

cc: Shailesh Virlley (Shailesh_Virlley@gensler.com)
Olivia Ku (Olivia_Ku@gensler.com)
David Coplon (dcoplon@martinmartin.com)
Jon Niemuth (Jon_Niemuth@gensler.com)
Aric Yarberry (AYarberry@lpc.com)
Aaron Van Dyke (AVanDyke@lpc.com)
Jay Hardy (jay@colmenagroup.com)
Robert De La Rosa (Robert_DeLaRosa@gensler.com)

POINT PHASE 1 PUBLIC INFRASTRUCTURE DISTRICT NO. 1
JOB NO. 4152-001-25



0.2 0 0 0 0 0 0.2 0.4 MI 0.6

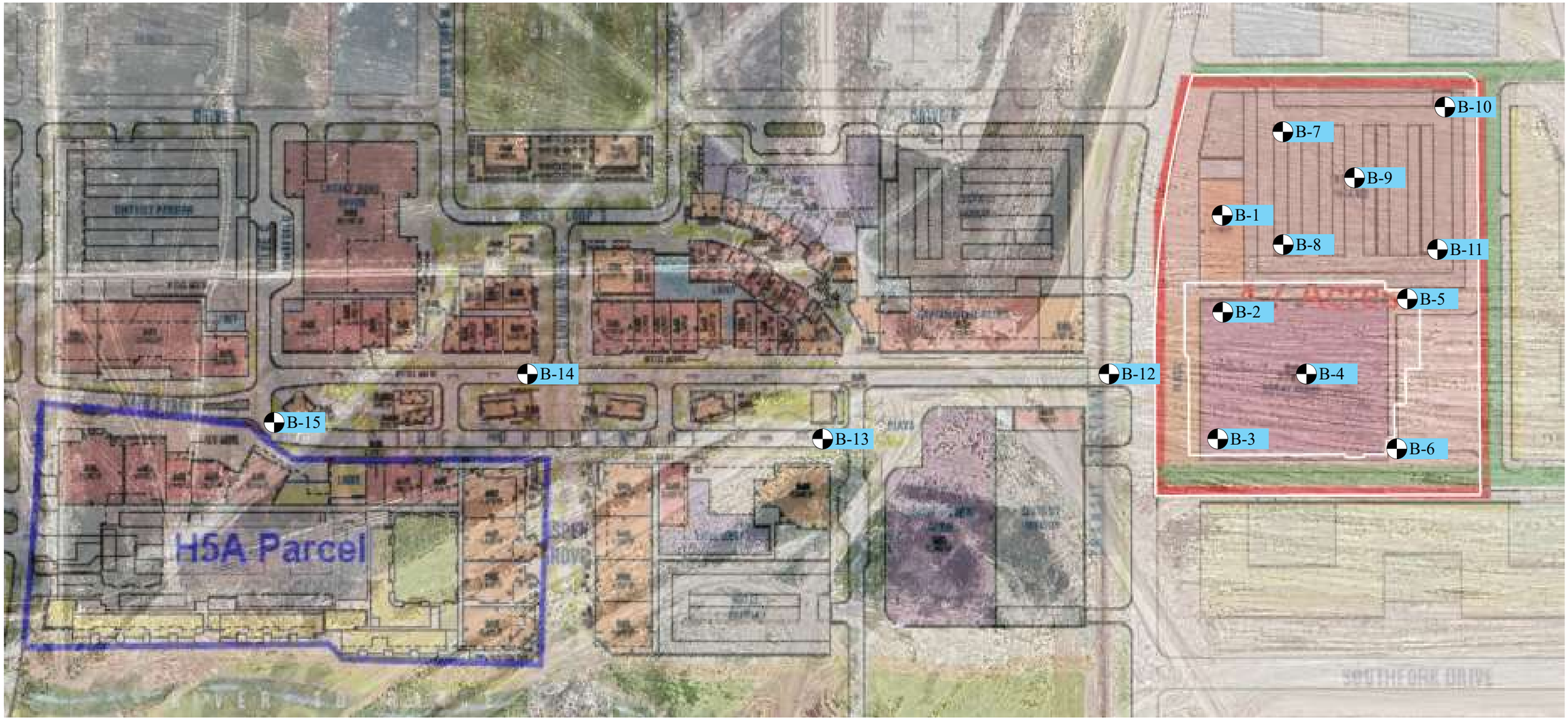
200 0 0 200 400 600 800 1000 M 1200

FIGURE 1
VICINITY MAP

REFERENCE:
ALL TRAILS - NATIONAL GEOGRAPHIC TERRAIN
DATED 2025

GSH

POINT PHASE 1 PUBLIC INFRASTRUCTURE DISTRICT NO. 1
JOB NO. 4152-001-25



REFERENCE:
ADAPTED FROM AERIAL PHOTOGRAPH
DOWNLOADED FROM GOOGLE EARTH
IMAGERY DATED 5/2025

APPROXIMATE SCALE IN FEET
80 0 80 160

FIGURE 2
SITE PLAN

GSH

BORING LOG

Page: 1 of 1

BORING: B-1

CLIENT: Point Phase 1 Public Infrastructure District No. 1 PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center DATE STARTED: 9/3/25 DATE FINISHED: 9/3/25

LOCATION: Approximately 14000 South 200 West, Draper, Utah (40.4934°, -111.8992°) GSH FIELD REP.: JA/JB

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger HAMMER: Automatic WEIGHT: 140 lbs DROP: 30"

GROUNDWATER DEPTH: Not Encountered (9/14/25) ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	CL	SILTY CLAY with major roots (topsoil) to 6"; brown									slightly moist loose/disturbed to 6"
		grades tan		30		6.8	69				very stiff
		grades fine sandy clay	5	29							
			10	19							
		grades brown									moist stiff
		grades with layers of fine to medium sand up to 1" thick; brown/ green	15	18		19.8	96				
		End of Exploration at 16.5'. No groundwater encountered at time of drilling. Installed 1.25" diameter slotted PVC pipe to 16.5'.	20								
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3A

BORING LOG

Page: 1 of 2

BORING: B-2

CLIENT: Point Phase 1 Public Infrastructure District No. 1 | PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center | DATE STARTED: 9/3/25 | DATE FINISHED: 9/3/25

LOCATION: Approximately 14000 South 200 West, Draper, Utah (40.4934°, -111.8992°) | GSH FIELD REP.: JB/JA

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger | HAMMER: Automatic | WEIGHT: 140 lbs | DROP: 30"

GROUNDWATER DEPTH: 28.0' (9/14/25) | ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	ML	CLAYEY SILT with some fine sand; brown									slightly moist loose/disturbed to 6"
	CL	SILTY CLAY moderately cemented; tan									slightly moist very stiff
				46							
			5	50/6"							hard
			10	44							
		grades brown									slightly moist very stiff
			15	23							
		grades brown/orange									moist
			20	45		23.2	99				
		grades fine sandy clay									
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3B

GSH

BORING LOG

Page: 2 of 2

BORING: B-2

CLIENT: Point Phase 1 Public Infrastructure District No. 1 PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center DATE STARTED: 9/3/25 DATE FINISHED: 9/3/25

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
			25								
		grades silty clay with fine to medium sand; grayish-brown		28							
▼											saturated
			30	18							
		grades brown									stiff
			35	39		27.8	95				
											very stiff
	SM	SILTY FINE TO MEDIUM SAND yellowish-brown									saturated medium dense
			40	61							
		End of Exploration at 41.5'. Installed 1.25" diameter slotted PVC pipe to 41.5'.									
			45								
			50								

See Subsurface Conditions section in the report for additional information.

FIGURE 3B
(continued)

BORING LOG

Page: 1 of 2

BORING: B-3

CLIENT: Point Phase 1 Public Infrastructure District No. 1 | PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center | DATE STARTED: 9/3/25 | DATE FINISHED: 9/3/25

LOCATION: Approximately 14000 South 200 West, Draper, Utah (40.4934°, -111.8992°) | GSH FIELD REP.: JB/JA

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger | HAMMER: Automatic | WEIGHT: 140 lbs | DROP: 30"

GROUNDWATER DEPTH: 29.0' (9/14/25) | ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	CL	FINE SANDY SILTY CLAY tan				2.4		69.8			slightly moist loose/disturbed to 6"
			5	57		12.9	94				stiff
		grades silty clay									
			10	33		18.7	92				
		grades with layers of sand up to 1/2" thick; gray	15	73							hard
			20	16							moist medium stiff
	SM	SILTY FINE TO MEDIUM SAND brown									moist dense
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3C

BORING LOG

Page: 2 of 2

BORING: B-3

CLIENT: Point Phase 1 Public Infrastructure District No. 1 PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center DATE STARTED: 9/3/25 DATE FINISHED: 9/3/25

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
			25								
				50/6"							very dense
▼	CL	SILTY CLAY gray									saturated stiff
			30								
				15		33.0	84				
		End of Exploration at 31.5'. No groundwater encountered at time of drilling. Installed 1.25" diameter slotted PVC pipe to 31.5.									
			35								
			40								
			45								
			50								

See Subsurface Conditions section in the report for additional information.

FIGURE 3C
(continued)

BORING LOG

Page: 1 of 3

BORING: B-4

CLIENT: Point Phase 1 Public Infrastructure District No. 1 PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center DATE STARTED: 9/4/25 DATE FINISHED: 9/4/25

LOCATION: Approximately 14000 South 200 West, Draper, Utah (40.4934°, -111.8992°) GSH FIELD REP.: JB/JA

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger HAMMER: Automatic WEIGHT: 140 lbs DROP: 30"

GROUNDWATER DEPTH: 27.0' (9/14/25) ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	CL	FINE SANDY CLAY brown									slightly moist loose/disturbed to 6"
		grades tan		27							very stiff
			5	79							hard
			10	41		18.4	100				very stiff
		grades silty clay with some fine sand	15	36					36	6	
			20	34							moist
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3D

BORING LOG

Page: 2 of 3

BORING: B-4

CLIENT: Point Phase 1 Public Infrastructure District No. 1 PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center DATE STARTED: 9/4/25 DATE FINISHED: 9/4/25

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
▼			25	23		33.2	88				stiff, saturated
			30	11		32.0	88				
			35	11					33	7	
			40	12		29.5	92				
			45	8							medium stiff
		grades dark gray/black	50	14							stiff

See Subsurface Conditions section in the report for additional information.

FIGURE 3D
(continued)

BORING LOG

Page: 3 of 3

BORING: B-4

CLIENT: Point Phase 1 Public Infrastructure District No. 1 PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center DATE STARTED: 9/4/25 DATE FINISHED: 9/4/25

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
			52								
		grades dark gray	55	11					44	7	
			60	18							
	SM	SILTY FINE TO MEDIUM SAND with trace fine gravel; brown	65	71							saturated medium dense
		grades silty fine sand; tan	70	52							
		End of Exploration at 71.5'. Installed 1.25" diameter slotted PVC pipe to 71.5'.	75								

See Subsurface Conditions section in the report for additional information.

FIGURE 3D
(continued)

BORING LOG

Page: 1 of 2

BORING: B-5

CLIENT: Point Phase 1 Public Infrastructure District No. 1 | PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center | DATE STARTED: 9/2/25 | DATE FINISHED: 9/2/25

LOCATION: Approximately 14000 South 200 West, Draper, Utah (40.4934°, -111.8992°) | GSH FIELD REP.: JA/JB

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger | HAMMER: Automatic | WEIGHT: 140 lbs | DROP: 30"

GROUNDWATER DEPTH: 23.5' (9/14/25) | ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	CL	SILTY CLAY with some fine sand; major roots (topsoil) to 6"; brown/tan									slightly moist loose/disturbed to 6"
				50/4"							hard
			5	84		10.0	83				
		grades fine sandy clay; brown	10	64							
		grades with layers of silty fine to medium sand up to 1/2"; brown/orange	15	42							moist very stiff
			20	18							stiff
▼											saturated
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3E

GSH

BORING LOG

Page: 2 of 2

BORING: B-5

CLIENT: Point Phase 1 Public Infrastructure District No. 1 PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center DATE STARTED: 9/2/25 DATE FINISHED: 9/2/25

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
			25								
		grades brown		27							very stiff
			30								
				24		34.6	86				
		End of Exploration at 31.5'. Installed 1.25" diameter slotted PVC pipe to 31.5'.	35								
			40								
			45								
			50								

See Subsurface Conditions section in the report for additional information.

FIGURE 3E
(continued)

BORING LOG

Page: 1 of 2

BORING: B-6

CLIENT: Point Phase 1 Public Infrastructure District No. 1 PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center DATE STARTED: 9/2/25 DATE FINISHED: 9/2/25

LOCATION: Approximately 14000 South 200 West, Draper, Utah (40.4934°, -111.8992°) GSH FIELD REP.: JA/JB

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger HAMMER: Automatic WEIGHT: 140 lbs DROP: 30"

GROUNDWATER DEPTH: 26.8' (9/14/25) ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	CL FILL	FINE TO COARSE SANDY SILTY CLAY, FILL brown									slightly moist loose/disturbed to 6"
	ML	CLAYEY SILT with some fine sand; tan		87		6.7		94.1			slightly moist hard
			5	86							
	CL	SILTY CLAY brown									slightly moist very stiff
			10	32							
			15	26		43.6	77				moist
			20	35		12.6	87	70.1			
		grades with fine sand; orange									
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3F

BORING LOG

Page: 2 of 2

BORING: B-6

CLIENT: Point Phase 1 Public Infrastructure District No. 1 PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center DATE STARTED: 9/2/25 DATE FINISHED: 9/2/25

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
▼		grades with layers of silty fine sand up to 1/2" thick; brown/orange	25	24							saturated
		sand grades out; gray	30	29		31.9	90				
			35	36							
			40	34		33.3	83				
			45	26		37.6	81				
			50	0		45.9	76				very soft
		End of Exploration at 51.5'. No groundwater encountered.									

See Subsurface Conditions section in the report for additional information.

FIGURE 3F
(continued)

BORING LOG

Page: 1 of 1

BORING: B-7

CLIENT: Point Phase 1 Public Infrastructure District No. 1 PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center DATE STARTED: 9/2/25 DATE FINISHED: 9/2/25

LOCATION: Approximately 14000 South 200 West, Draper, Utah (40.4934°, -111.8992°) GSH FIELD REP.: JB/JA

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger HAMMER: Automatic WEIGHT: 140 lbs DROP: 30"

GROUNDWATER DEPTH: Not Encountered (9/2/25) ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	ML	CLAYEY SILT with some fine sand; tan									slightly moist loose/disturbed to 6"
	CL	SILTY CLAY tan	5								slightly moist soft
		End of Exploration at 5.5'. No groundwater encountered at time of drilling.	10								
			15								
			20								
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3G

BORING LOG

Page: 1 of 1

BORING: B-8

CLIENT: Point Phase 1 Public Infrastructure District No. 1 PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center DATE STARTED: 9/3/25 DATE FINISHED: 9/3/25

LOCATION: Approximately 14000 South 200 West, Draper, Utah (40.4934°, -111.8992°) GSH FIELD REP.: JA/JB

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger HAMMER: Automatic WEIGHT: 140 lbs DROP: 30"

GROUNDWATER DEPTH: Not Encountered (9/3/25) ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	ML	CLAYEY SILT with some fine sand; brown									slightly moist loose/disturbed to 6"
	CL	SILTY CLAY brown	5								slightly moist soft
		End of Exploration at 5.5'. No groundwater encountered at time of drilling.	10								
			15								
			20								
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3H

BORING LOG

Page: 1 of 1

BORING: B-9

CLIENT: Point Phase 1 Public Infrastructure District No. 1 PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center DATE STARTED: 9/3/25 DATE FINISHED: 9/3/25

LOCATION: Approximately 14000 South 200 West, Draper, Utah (40.4934°, -111.8992°) GSH FIELD REP.: JA/JB

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger HAMMER: Automatic WEIGHT: 140 lbs DROP: 30"

GROUNDWATER DEPTH: Not Encountered (9/3/25) ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	CL	SILTY CLAY with some fine sand; tan									slightly moist loose/disturbed to 6"
			5								soft
		End of Exploration at 5.5'. No groundwater encountered at time of drilling.									
			10								
			15								
			20								
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3I

BORING LOG

Page: 1 of 1

BORING: B-10

CLIENT: Point Phase 1 Public Infrastructure District No. 1 PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center DATE STARTED: 9/3/25 DATE FINISHED: 9/3/25

LOCATION: Approximately 14000 South 200 West, Draper, Utah (40.4934°, -111.8992°) GSH FIELD REP.: JB/JA

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger HAMMER: Automatic WEIGHT: 140 lbs DROP: 30"

GROUNDWATER DEPTH: Not Encountered (9/3/25) ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	CL	SILTY CLAY well cemented; tan									slightly moist loose/disturbed to 6"
			5			22.1		97.5			soft
		End of Exploration at 5.5'. No groundwater encountered at time of drilling.									
			10								
			15								
			20								
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3J

BORING LOG

Page: 1 of 1

BORING: B-11

CLIENT: Point Phase 1 Public Infrastructure District No. 1 PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center DATE STARTED: 9/3/25 DATE FINISHED: 9/3/25

LOCATION: Approximately 14000 South 200 West, Draper, Utah (40.4934°, -111.8992°) GSH FIELD REP.: JB/JA

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger HAMMER: Automatic WEIGHT: 140 lbs DROP: 30"

GROUNDWATER DEPTH: Not Encountered (9/3/25) ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	CL	FINE SANDY SILT tan									slightly moist loose/disturbed to 6"
		grades with some clay	5								
		End of Exploration at 5.5'. No groundwater encountered at time of drilling.									
			10								
			15								
			20								
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3K

BORING LOG

Page: 1 of 1

BORING: B-12

CLIENT: Point Phase 1 Public Infrastructure District No. 1 PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center DATE STARTED: 9/3/25 DATE FINISHED: 9/3/25

LOCATION: Approximately 14000 South 200 West, Draper, Utah (40.4934°, -111.8992°) GSH FIELD REP.: JB/JA

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger HAMMER: Automatic WEIGHT: 140 lbs DROP: 30"

GROUNDWATER DEPTH: Not Encountered (9/3/25) ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	CL	FINE SANDY SILTY CLAY with some fine and coarse gravel; brown									slightly moist loose/disturbed to 6"
		grades clayey silt with some fine sand; tan	5								stiff
		End of Exploration at 5.5'. No groundwater encountered at time of drilling.									
			10								
			15								
			20								
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3L

BORING LOG

Page: 1 of 1

BORING: B-13

CLIENT: Point Phase 1 Public Infrastructure District No. 1 PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center DATE STARTED: 9/3/25 DATE FINISHED: 9/3/25

LOCATION: Approximately 14000 South 200 West, Draper, Utah (40.4934°, -111.8992°) GSH FIELD REP.: JA/JB

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger HAMMER: Automatic WEIGHT: 140 lbs DROP: 30"

GROUNDWATER DEPTH: Not Encountered (9/3/25) ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	ML	CLAYEY SILT with some fine sand; tan									slightly moist loose/disturbed to 6"
	CL	SILTY CLAY with some fine sand; tan									slightly moist very stiff
				31		40.8	74				
			5	88							hard
			10	38		6.8	78				very stiff
		End of Exploration at 11.5'. No groundwater encountered at time of drilling. Installed 1.25" diameter slotted PVC pipe to 11.5'.	15								
			20								
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3M

BORING LOG

Page: 1 of 1

BORING: B-14

CLIENT: Point Phase 1 Public Infrastructure District No. 1 PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center DATE STARTED: 9/3/25 DATE FINISHED: 9/3/25

LOCATION: Approximately 14000 South 200 West, Draper, Utah (40.4934°, -111.8992°) GSH FIELD REP.: JB/JA

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger HAMMER: Automatic WEIGHT: 140 lbs DROP: 30"

GROUNDWATER DEPTH: Not Encountered (9/3/25) ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	ML	CLAYEY SILT with some fine sand; brown									slightly moist loose/disturbed to 6"
		grades tan	5								
		End of Exploration at 5.5'. No groundwater encountered at time of drilling.									
			10								
			15								
			20								
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3N

BORING LOG

Page: 1 of 1

BORING: B-15

CLIENT: Point Phase 1 Public Infrastructure District No. 1 PROJECT NUMBER: 4152-001-25

PROJECT: The Point Event Center DATE STARTED: 9/3/25 DATE FINISHED: 9/3/25

LOCATION: Approximately 14000 South 200 West, Draper, Utah (40.4934°, -111.8992°) GSH FIELD REP.: JB/JA

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger HAMMER: Automatic WEIGHT: 140 lbs DROP: 30"

GROUNDWATER DEPTH: Not Encountered (9/3/25) ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	ML	CLAYEY SILT with some fine sand; brown									slightly moist loose/disturbed to 6"
		grades tan		41							very stiff
	CL	FINE SANDY CLAY with silt; tan	5	45		10.5	92				slightly moist very stiff
		grades brown	10	41							
		End of Exploration at 11.5'. No groundwater encountered at time of drilling.	15								
			20								
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3O

CLIENT: Point Phase 1 Public Infrastructure District No. 1
PROJECT: The Point Event Center
PROJECT NUMBER: 4152-001-25

KEY TO BORING LOG

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
①	②	③	④	⑤	⑥	⑦	⑧	⑨	⑩	⑪	⑫

COLUMN DESCRIPTIONS

① **Water Level:** Depth to measured groundwater table. See symbol below.

② **USCS:** (Unified Soil Classification System) Description of soils encountered; typical symbols are explained below.

③ **Description:** Description of material encountered; may include color, moisture, grain size, density/consistency,

④ **Depth (ft.):** Depth in feet below the ground surface.

⑤ **Blow Count:** Number of blows to advance sampler 12" beyond first 6", using a 140-lb hammer with 30" drop.

⑥ **Sample Symbol:** Type of soil sample collected at depth interval shown; sampler symbols are explained below.

⑦ **Moisture (%):** Water content of soil sample measured in laboratory; expressed as percentage of dryweight of

⑧ **Dry Density (pcf):** The density of a soil measured in laboratory; expressed in pounds per cubic foot.

⑨ **% Passing 200:** Fines content of soils sample passing a No. 200 sieve; expressed as a percentage.

⑩ **Liquid Limit (%):** Water content at which a soil changes from plastic to liquid behavior.

⑪ **Plasticity Index (%):** Range of water content at which a soil exhibits plastic properties.

⑫ **Remarks:** Comments and observations regarding drilling or sampling made by driller or field personnel. May include other field and laboratory test results using the following abbreviations:

CEMENTATION:

Weakly: Crumbles or breaks with handling or slight finger pressure.

Moderately: Crumbles or breaks with considerable finger pressure.

Strongly: Will not crumble or break with finger pressure.

MODIFIERS:

Trace <5%

Some 5-12%

With > 12%

MOISTURE CONTENT (FIELD TEST):

Dry: Absence of moisture, dusty, dry to the touch.

Moist: Damp but no visible water.

Saturated: Visible water, usually soil below water table.

Descriptions and stratum lines are interpretive; field descriptions may have been modified to reflect lab test results. Descriptions on the logs apply only at the specific boring locations and at the time the borings were advanced; they are not warranted to be representative of subsurface conditions at other locations or times.

UNIFIED SOIL CLASSIFICATION SYSTEM (USCS)

MAJOR DIVISIONS			USCS SYMBOLS	TYPICAL DESCRIPTIONS
COARSE-GRAINED SOILS More than 50% of material is larger than No. 200 sieve size.	GRAVELS More than 50% of coarse fraction retained on No. 4 sieve.	CLEAN GRAVELS (little or no fines)	GW	Well-Graded Gravels, Gravel-Sand Mixtures, Little or No Fines
			GP	Poorly-Graded Gravels, Gravel-Sand Mixtures, Little or No Fines
		GRAVELS WITH FINES (appreciable amount of fines)	GM	Silty Gravels, Gravel-Sand-Silt Mixtures
			GC	Clayey Gravels, Gravel-Sand-Clay Mixtures
	SANDS More than 50% of coarse fraction passing through No. 4 sieve.	CLEAN SANDS (little or no fines)	SW	Well-Graded Sands, Gravelly Sands, Little or No Fines
			SP	Poorly-Graded Sands, Gravelly Sands, Little or No Fines
		SANDS WITH FINES (appreciable amount of fines)	SM	Silty Sands, Sand-Silt Mixtures
			SC	Clayey Sands, Sand-Clay Mixtures
FINE-GRAINED SOILS More than 50% of material is smaller than No. 200 sieve size.	SILTS AND CLAYS Liquid Limit less than 50%		ML	Inorganic Silts and Very Fine Sands, Rock Flour, Silty or Clayey Fine Sands or Clayey Silts with Slight Plasticity
			CL	Inorganic Clays of Low to Medium Plasticity, Gravelly Clays, Sandy Clays, Silty Clays, Lean Clays
			OL	Organic Silts and Organic Silty Clays o f Low Plasticity
	SILTS AND CLAYS Liquid Limit greater than 50%		MH	Inorganic Silts, Micacious or Diatomacious Fine Sand or Silty Soils
			CH	Inorganic Clays of High Plasticity, Fat Clays
			OH	Organic Silts and Organic Clays of Medium to High Plasticity
HIGHLY ORGANIC SOILS			PT	Peat, Humus, Swamp Soils with High Organic Contents

Note: Dual Symbols are used to indicate borderline soil classifications.

STRATIFICATION:

DESCRIPTION	THICKNESS
Seam	up to 1/8"
Layer	1/8" to 12"
Occasional: One or less per 6" of thickness	
Numerous; More than one per 6" of thickness	

TYPICAL SAMPLER GRAPHIC SYMBOLS

- Bulk/Bag Sample
- Standard Penetration Split Spoon Sampler
- Rock Core
- No Recovery
- 3.25" OD, 2.42" ID D&M Sampler
- 3.0" OD, 2.42" ID D&M Sampler
- California Sampler
- Thin Wall

WATER SYMBOL

Water Level

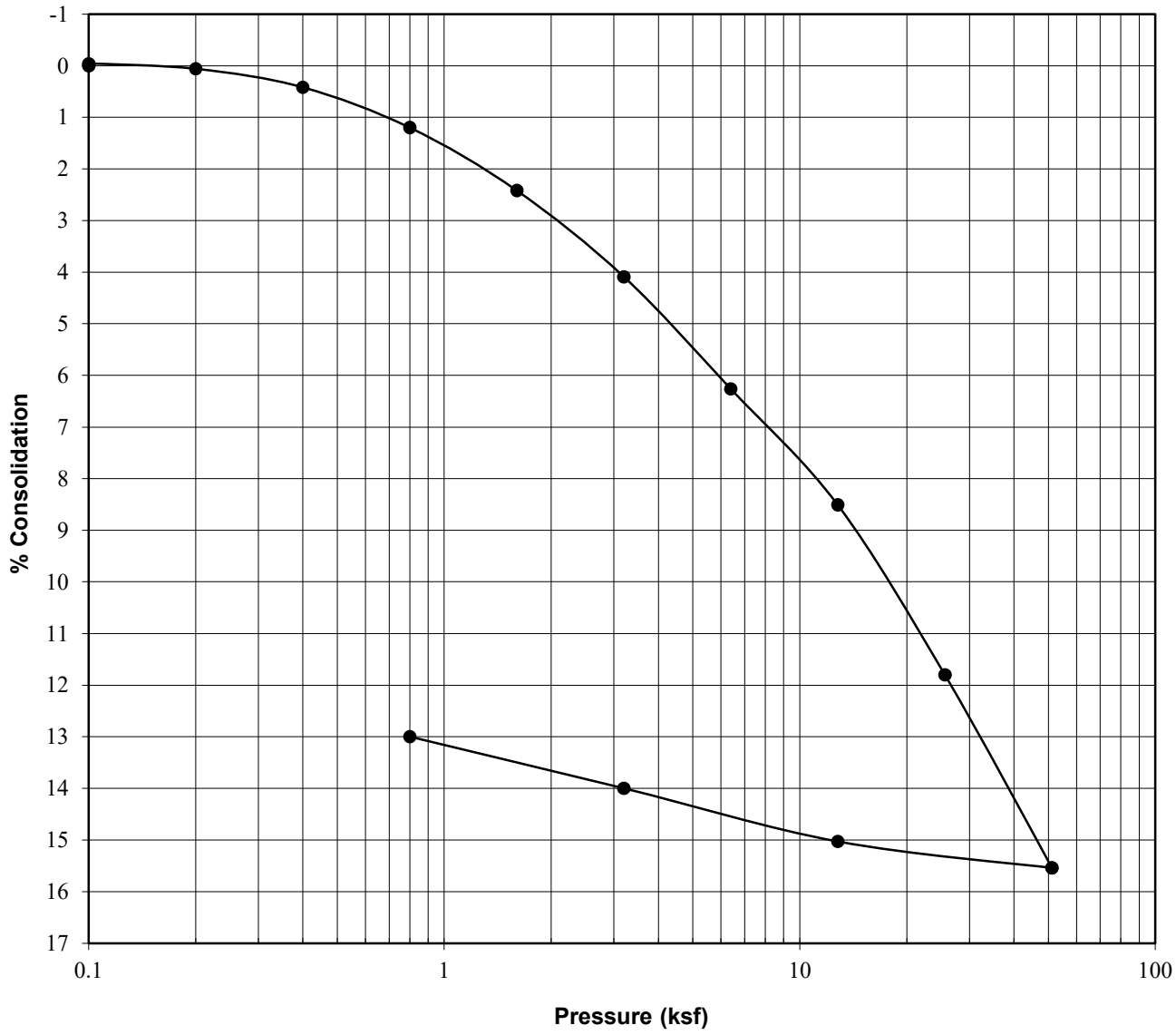
FIGURE 4

GSH

Attachment 1

Bearing Capacity and Settlement Calculations

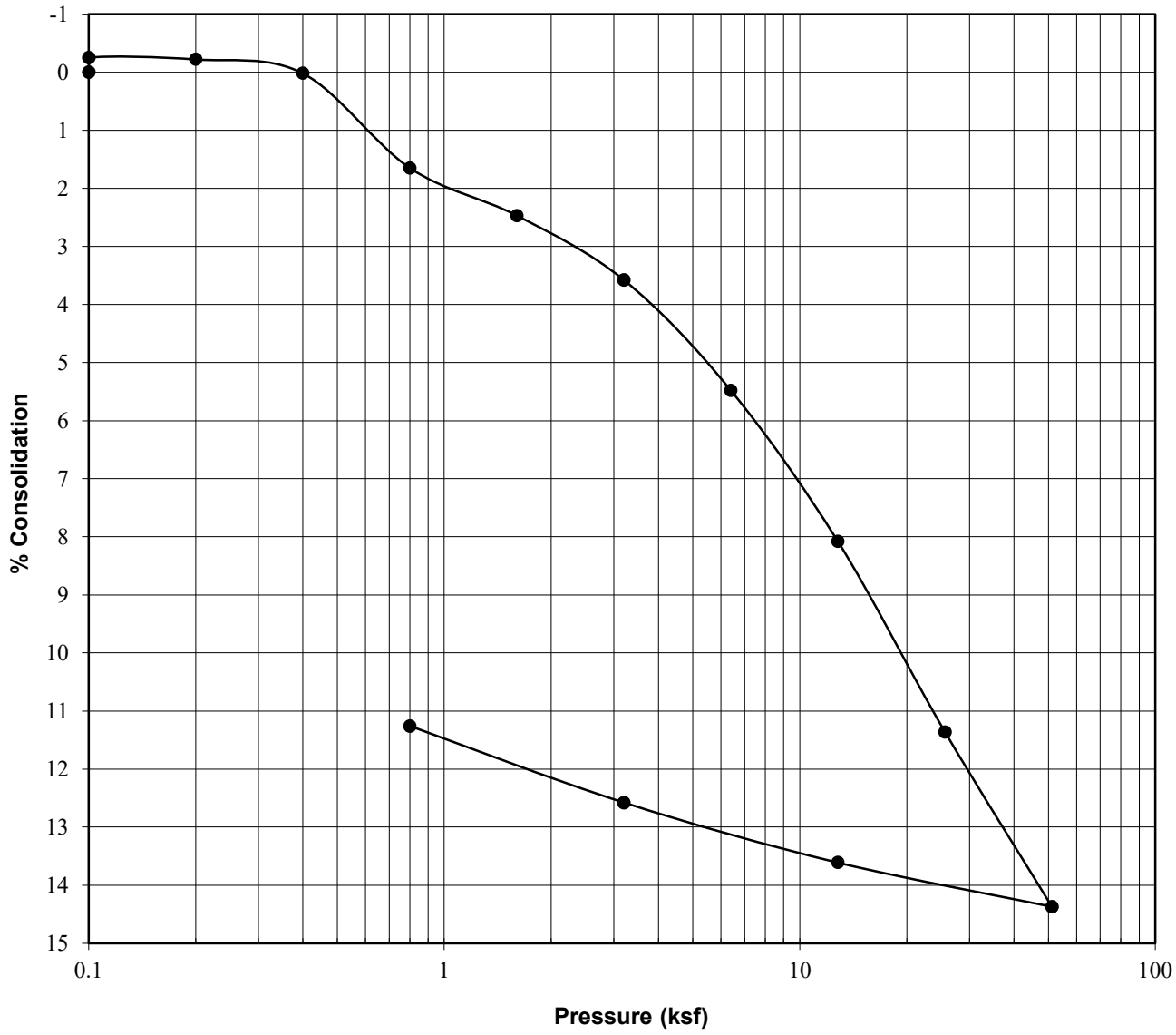
CONSOLIDATION TEST



Project:	The Point Event Center		
Location:	B1		
Sample Depth:	5	**Natural Moisture, %:**	25
Description:	Rings	**Dry Density, pcf:**	80
Water Added at:	0.1 ksf	**Liquid Limit:**	N/A
Percent Swell:	0.0	**Plasticity Index:**	N/A
Soil Type:	Tan Fine Sandy Silty Clay		

PROJECT NO.: 4152-001-25	GSH	**FIGURE NO.:**

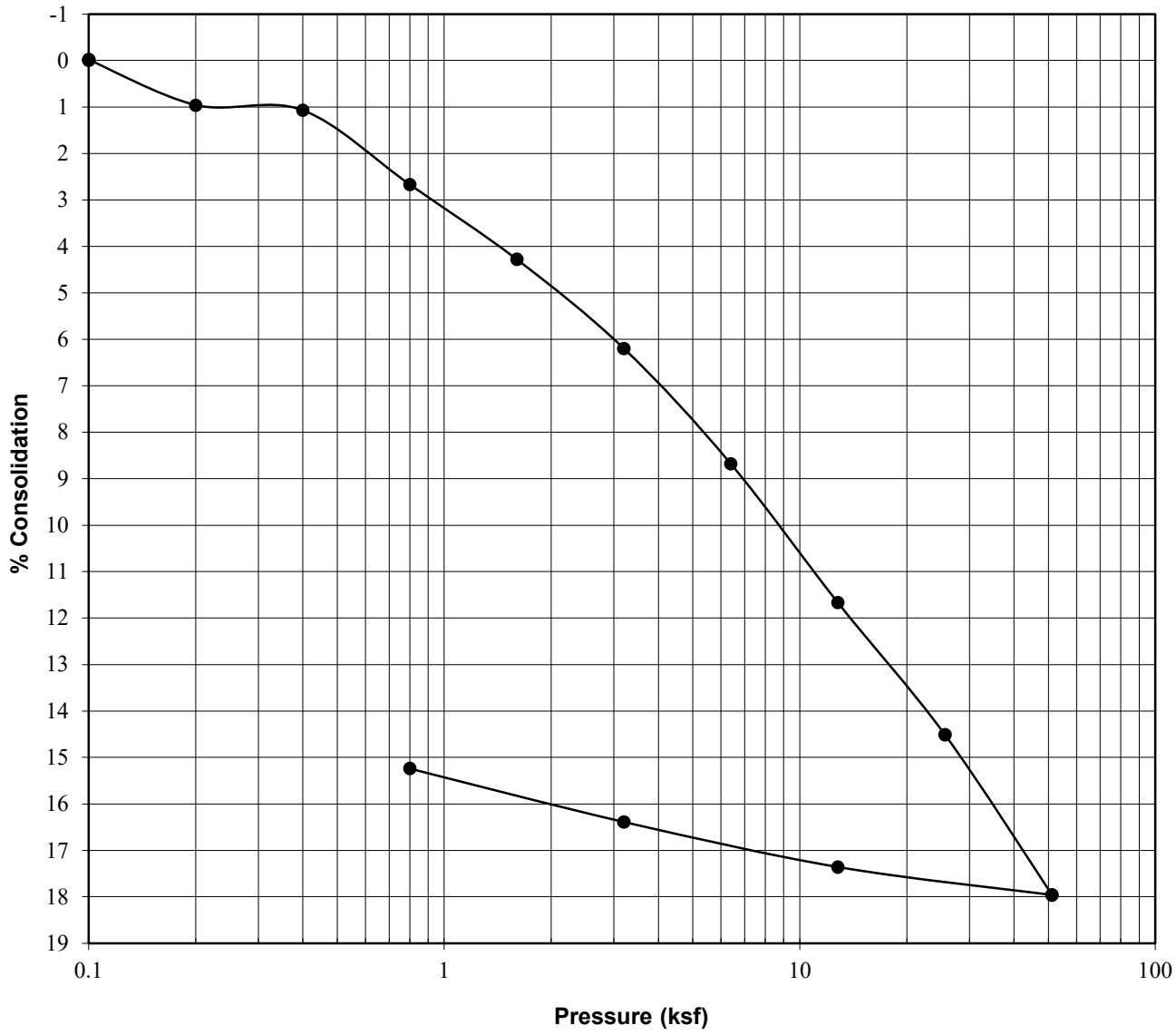
CONSOLIDATION TEST



Project:	The Point Event Center		
Location:	B1		
Sample Depth:	10	**Natural Moisture, %:**	24
Description:	Rings	**Dry Density, pcf:**	96
Water Added at:	0.1 ksf	**Liquid Limit:**	N/A
Percent Swell:	0.3	**Plasticity Index:**	N/A
Soil Type:	BRN FSA SiCL		

PROJECT NO.: 4152-001-25	GSH	**FIGURE NO.:**

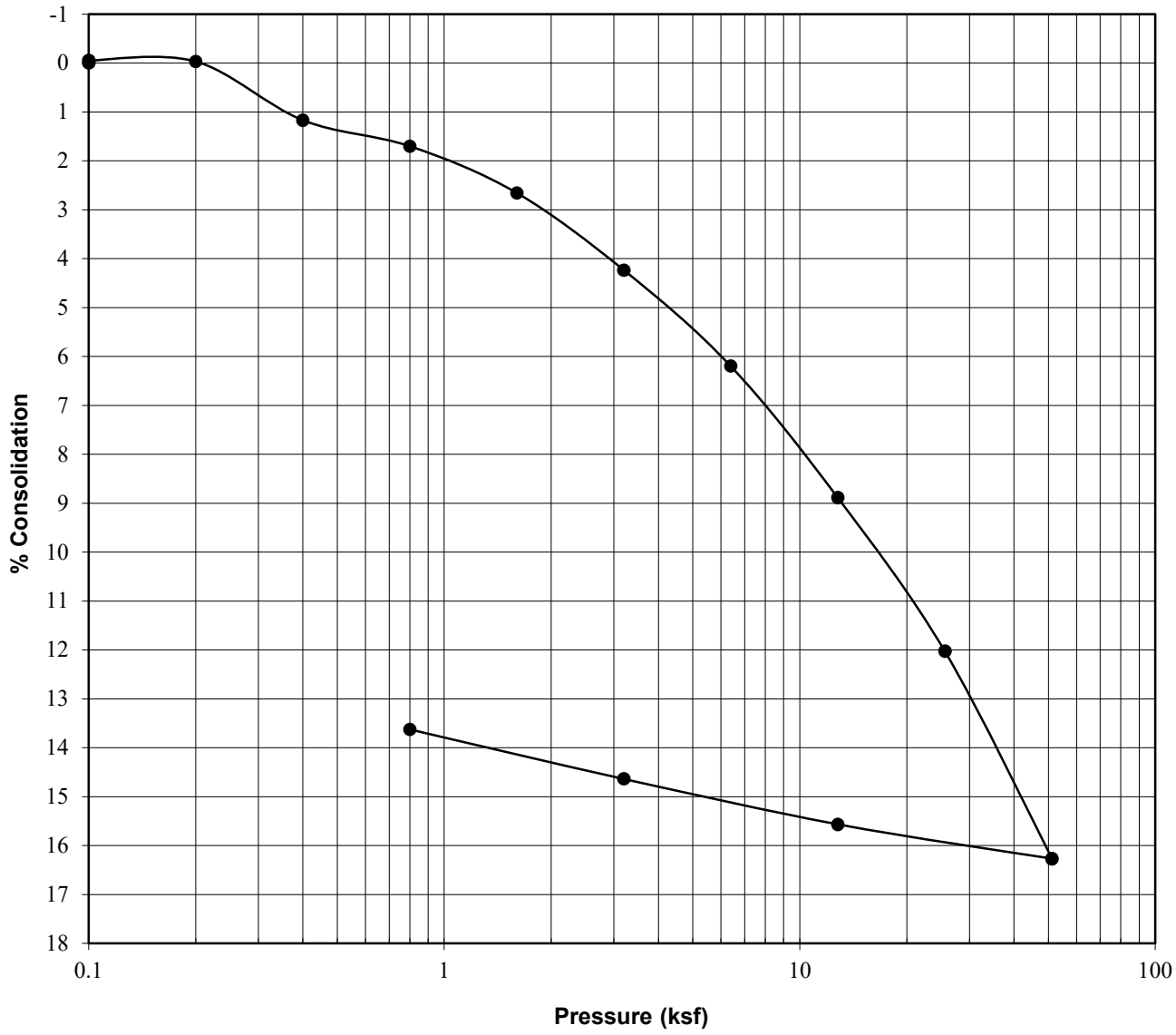
CONSOLIDATION TEST



Project:	The Point Event Center		
Location:	B2		
Sample Depth:	15	**Natural Moisture, %:**	24
Description:	Rings	**Dry Density, pcf:**	96
Water Added at:	0.1 ksf	**Liquid Limit:**	N/A
Percent Swell:	0.0	**Plasticity Index:**	N/A
Soil Type:	Gray Fine Sandy Silty Clay		

PROJECT NO.: 4152-001-25	GSH	**FIGURE NO.:**

CONSOLIDATION TEST



Project:	The Point Event Center		
Location:	B5		
Sample Depth:	20	**Natural Moisture, %:**	27
Description:	Rings	**Dry Density, pcf:**	88
Water Added at:	0.1 ksf	**Liquid Limit:**	N/A
Percent Swell:	0.1	**Plasticity Index:**	N/A
Soil Type:	Fine Sandy Silty Clay		

PROJECT NO.: 4152-001-25	GSH	**FIGURE NO.:**

The Point Event Center 4152-001-25 TJL 9/22/2025

Square Foundation

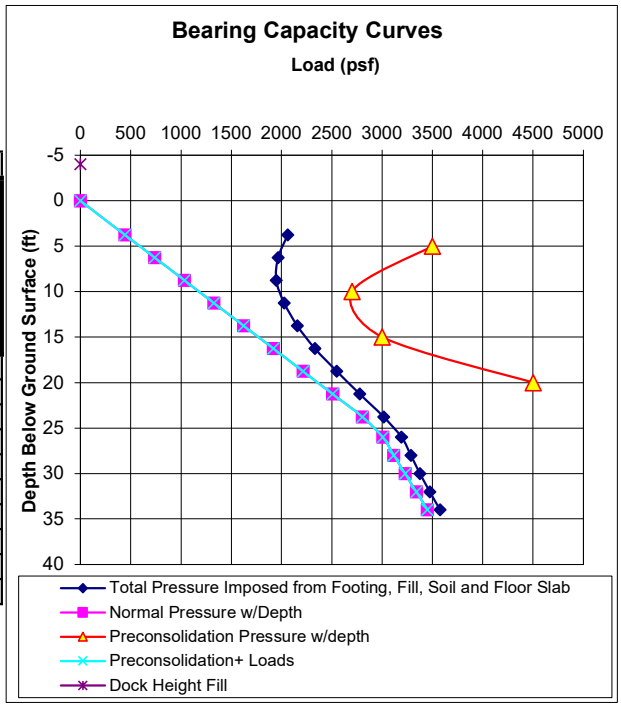
Depth of Footing (ft)= 1.5
Depth of Water (ft) = 25 Note if water table was not encountered this number has to be greater than the maximum depth you are calculating pressures for.

Assumed Bearing Capacity BC= 2000 psf
Column Load L= 400 kips
Width of Footing b= 14.14 feet
Unit Weight γ= 118 pcf

Depth Below Ground Surface	Average Depth Below Ground Surface	Average Depth Below Found * D	D/ width of Found	Influence of found load (from table)	P'o	ΔP+P'o	(ΔP+P'o) / P'o	Log ( )	Cc	Thickness of Depth Increment	Unit Settlement	Total Settlement
Feet	Feet	Feet		%	PSF	PSF				Inches	Inches	Inches
0.0	0.0	0.0	0.00	0.00	0	0	0.00	0.000	0.003	30.0	0.00	0.00
2.5	3.8	2.3	0.16	0.81	443	2063	4.66	0.668	0.003	30.0	0.06	0.06
5.0	6.3	4.8	0.34	0.62	738	1968	2.67	0.426	0.017	30.0	0.22	0.28
7.5	8.8	7.3	0.51	0.46	1033	1948	1.89	0.276	0.017	30.0	0.14	0.42
10.0	11.3	9.8	0.69	0.35	1328	2028	1.53	0.184	0.022	30.0	0.12	0.54
12.5	13.8	12.3	0.87	0.27	1623	2158	1.33	0.124	0.022	30.0	0.08	0.62
15.0	16.3	14.8	1.04	0.21	1918	2331	1.22	0.085	0.019	30.0	0.05	0.67
17.5	18.8	17.3	1.22	0.17	2213	2549	1.15	0.062	0.019	30.0	0.04	0.70
20.0	21.3	19.8	1.40	0.13	2508	2777	1.11	0.044	0.017	30.0	0.02	0.73
22.5	23.8	22.3	1.57	0.11	2803	3014	1.08	0.032	0.017	30.0	0.02	0.74
25.0	26.0	24.5	1.73	0.09	3006	3193	1.06	0.026	0.017	24.0	0.01	0.75
27.0	28.0	26.5	1.87	0.08	3117	3286	1.05	0.023	0.017	24.0	0.01	0.76
29.0	30.0	28.5	2.02	0.07	3228	3377	1.05	0.020	0.017	24.0	0.01	0.77
31.0	32.0	30.5	2.16	0.07	3339	3474	1.04	0.017	0.017	24.0	0.01	0.78
33.0	34.0	32.5	2.30	0.06	3450	3575	1.04	0.015	0.017	24.0	0.01	0.78
35.0										Total Settlement	0.78	Inches

Preload 0 psf
Floorslab 0 psf

δP+P'o+Loads PSF	Average Depth Below Ground Surface Feet	P'o PSF	P'o + Loads PSF	Average Depth Below Ground Surface Feet	Preconsolidation Pressures PSF	Depth Feet
0	-4					
0	0	0	0	0	3,500	5
2063	3.75	443	442.5	3.75	2,700	10
1968	6.25	738	737.5	6.25	3000	15
1948	8.75	1033	1032.5	8.75	4500	20
2028	11.25	1328	1327.5	11.25		
2158	13.75	1623	1622.5	13.75		
2331	16.25	1918	1917.5	16.25		
2549	18.75	2213	2212.5	18.75		
2777	21.25	2508	2507.5	21.25		
3014	23.75	2803	2802.5	23.75		
3193	26	3006	3005.6	26		
3286	28	3117	3116.8	28		
3377	30	3228	3228	30		
3474	32	3339	3339.2	32		
3575	34	3450	3450.4	34		
0	0	0	0	0		
0	0	0	0	0		



The Point Event Center | 4152-001-25 | TJL | 9/22/2025

Strip Foundation

Depth of Footing (ft)= 2.5
Depth of Water (ft) = 25 Note if water table was not encountered this number has to be greater than the maximum depth you are calculating pressures for.

Assumed Bearing Capacity BC= 2000 psf
Wall Load L= 12 kips/ft
Width of Footing b= 4.00 feet
Unit Weight γ= 118 pcf

Depth Below Ground Surface	Average Depth Below Ground Surface	Average Depth Below Found * D	D/ width of Found	Influence of found load (from table)	P'o	ΔP+P'o	(ΔP+P'o) / P'o	Log ( )	Cc	Thickness of Depth Increment	Unit Settlement	Total Settlement
Feet	Feet	Feet		%	PSF	PSF				Inches	Inches	Inches
0.0	0.0	0.0	0.00	0.00	0	0	0.00	0.000	0.003	30.0	0.00	0.00
2.5	3.8	1.3	0.31	0.74	443	1922	4.34	0.638	0.003	30.0	0.06	0.06
5.0	6.3	3.8	0.94	0.41	738	1564	2.12	0.327	0.017	30.0	0.17	0.22
7.5	8.8	6.3	1.56	0.27	1033	1576	1.53	0.184	0.017	30.0	0.09	0.32
10.0	11.3	8.8	2.19	0.21	1328	1744	1.31	0.118	0.022	30.0	0.08	0.40
12.5	13.8	11.3	2.81	0.16	1623	1938	1.19	0.077	0.022	30.0	0.05	0.45
15.0	16.3	13.8	3.44	0.13	1918	2180	1.14	0.056	0.019	30.0	0.03	0.48
17.5	18.8	16.3	4.06	0.11	2213	2437	1.10	0.042	0.019	30.0	0.02	0.50
20.0	21.3	18.8	4.69	0.10	2508	2702	1.08	0.032	0.017	30.0	0.02	0.52
22.5	23.8	21.3	5.31	0.08	2803	2969	1.06	0.025	0.017	30.0	0.01	0.53
25.0	26.0	23.5	5.88	0.08	3006	3157	1.05	0.021	0.017	24.0	0.01	0.54
27.0	28.0	25.5	6.38	0.07	3117	3254	1.04	0.019	0.017	24.0	0.01	0.55
29.0	30.0	27.5	6.88	0.06	3228	3355	1.04	0.017	0.017	24.0	0.01	0.55
31.0	32.0	29.5	7.38	0.06	3339	3458	1.04	0.015	0.017	24.0	0.01	0.56
33.0	34.0	31.5	7.88	0.06	3450	3562	1.03	0.014	0.017	24.0	0.01	0.57
										Total settlement	0.57	Inches

35

Preload 0 psf
Floorslab 0 psf

δP+P'o+Loads	Average Depth Below Ground Surface	P'o	P'o + Loads	Average Depth Below Ground Surface	Preconsolidation Pressures	Depth
PSF	Feet	PSF	PSF	Feet	PSF	Feet
0	-4					
0	0	0	0	0	3,500	5
1922	3.75	443	442.5	3.75	2,700	10
1564	6.25	738	737.5	6.25	3000	15
1576	8.75	1033	1032.5	8.75	4500	20
1744	11.25	1328	1327.5	11.25		
1938	13.75	1623	1622.5	13.75		
2180	16.25	1918	1917.5	16.25		
2437	18.75	2213	2212.5	18.75		
2702	21.25	2508	2507.5	21.25		
2969	23.75	2803	2802.5	23.75		
3157	26	3006	3005.6	26		
3254	28	3117	3116.8	28		
3355	30	3228	3228	30		
3458	32	3339	3339.2	32		
3562	34	3450	3450.4	34		
0	0	0	0	0		
0	0	0	0	0